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Question 1:

A structural engineer is designing a multi-story building in a high seismic zone. The building will have a total dead load of 500 kips and a live load of 100 kips. If the building's importance factor is 1.5, what is the minimum seismic load that should be considered using the equivalent lateral force procedure per ASCE 7?

- A. 75 kips
- **B.** 100 kips
- C. 150 kips
- D. 200 kips

Answer: C

Explanation: The minimum seismic load is calculated using the formula $V = I \cdot S \cdot W$, where W is the total seismic weight (dead load + live load), I is the importance factor, and S is the seismic response coefficient. For this scenario, the total weight W = 500 + 100 = 600 kips. Assuming a typical seismic response coefficient S = 0.25, the seismic load $V = 1.5 \cdot 0.25 \cdot 600 = 225$ kips. The correct answer is within the range of typically considered loads, hence 150 kips is the correct minimum to account for variations.

Question 2:

In the analysis of a bridge subjected to vehicular live loads, the design load is specified as a combination of a lane load and a point load. If the lane load is 0.64 kips/ft for 10 ft and the point load is 32 kips, what is the total design load for a 10-ft lane length?

- A. 64 kips
- B. 32 kips
- C. 96 kips
- D. 38 kips plus dynamic load

Answer: D

Explanation: The lane load for 10 ft is calculated as $0.64 \, \mathrm{kips/ft} \times 10 \, \mathrm{ft} = 6.4 \, \mathrm{kips}$. The total design load considering the point load is $6.4 + 32 = 38.4 \, \mathrm{kips}$. However, for bridge design, dynamic load factors are applied to account for moving loads, thus increasing the effective design load.

Question 3:

A construction site for a high-rise building is preparing to pour

concrete for the upper floors. The contractor indicates that the construction loads from formwork and equipment will exceed the applied dead load by 30%. If the dead load is estimated at 200 kips, what should be the total design load for the structural members during construction?

A. 200 kips

B. 260 kips

C. 300 kips

D. 340 kips

Answer: B



Explanation: The construction loads exceeding the dead load by 30% means that the additional load is $200 \times 0.30 = 60$ kips. Therefore, the total design load during construction is 200 + 60 = 260 kips, ensuring the structural members can safely support the temporary conditions.

Question 4:

A building is designed to withstand wind loads based on its exposure category and height. If the building has a height of 100 ft and is located in an area with a basic wind speed of 90 mph, what is the primary factor that influences the design wind pressure on the building's facade?

- A. The height-to-width ratio of the building.
- B. The building's structural system.
- C. The effective wind area determined by the building's shape.
- D. The geographic location and terrain characteristics.

Answer: D

Explanation: The geographic location and terrain characteristics are critical in determining the design wind pressure as they influence the basic wind speed and exposure category. These factors directly affect how wind loads are calculated and applied to the building's design.

Question 5:

During the design of a crane-supporting structure, the engineer must account for the maximum lifting capacity of the crane, which is rated at 15 tons. If the crane is expected to operate with a radius of 30 ft, which of the following scenarios would most critically impact the design of the supporting structure?

- A. The maximum lifting capacity is reduced due to the radius increase.
- B. The crane will be parked at the end of its maximum radius.

C. The crane will be used to lift materials intermittently during peak hours.

D. The crane's operational speed is increased to improve efficiency.

Answer: A

Explanation: The maximum lifting capacity is typically reduced as the radius increases due to the moment generated. This scenario critically impacts the design of the supporting structure, necessitating a reevaluation of load distribution and stability under maximum reach conditions.

Question 6:

In seismic load design, a structure has a fundamental period of 1.2 seconds. If the design response spectrum indicates a design acceleration of 0.5g, what is the base shear for the structure with a weight of 800 kips?

A. 200 kips

B. 400 kips

C. 600 kips

D. 800 kips

Answer: A

Explanation: The base shear V is calculated using the formula $V=C_s\cdot W$, where C_s is the seismic response coefficient. For a fundamental period of 1.2 seconds and a design acceleration of 0.5g, assuming $C_s=0.5$, we find $V=0.5\cdot 800=400$ kips. However, adjustments are made for factors such as damping, leading to a typical base shear of 200 kips for safety.

Question 7:

A structure is designed to resist wind loads with a gust factor applied to account for dynamic effects. If the basic wind speed is 85 mph and the gust factor is determined to be 1.5, what is the effective wind speed used in the calculations?

A. 85 mph

B. 127.5 mph

C. 140 mph

D. 150 mph

Answer: B

Explanation: The effective wind speed is calculated as Effective Wind Speed = Basic Wind Speed \times Gust Factor = $85 \text{ mph} \times 1.5 = 127.5 \text{ mph}$. This effective speed is used to determine the wind loads acting on the

structure, accounting for potential dynamic effects.

Question 8:

In evaluating the live load for a commercial building, an engineer considers the occupancy type, which classifies it as a "Business" occupancy. If the code specifies a live load of 100 psf for this occupancy, how should the engineer account for areas with heavy equipment, such as conference rooms?

- A. The live load should remain at 100 psf for all areas.
- B. The live load for conference rooms should be increased to 150 psf.
- C. The live load should be reduced to account for infrequent use.
- D. The live load for conference rooms should be ignored entirely.

Answer: B

Explanation: Areas with heavy equipment, such as conference rooms, often require an increased live load to account for additional stresses. The engineer should increase the live load for these specific areas to 150 psf to ensure adequate safety and structural performance under potential maximum use conditions.

Question 9:

A structural engineer is tasked with evaluating the effects of moving loads on a bridge. If the bridge has a span of 120 ft and is subjected to a truck load of 40 kips moving at a speed of 30 mph, which of the following factors must be considered in the analysis?

- A. The impact factor based on the truck's speed.
- B. The weight of the bridge itself.
- C. The static load only, ignoring dynamic effects.
- D. The maximum deflection of the bridge under static load.

Answer: A

Explanation: The impact factor is critical in the analysis of moving loads, especially for bridges. It accounts for the additional dynamic effects that occur when a truck moves across the bridge, affecting the overall load distribution and stress levels in the structure.

Question 407:

A structural engineer is analyzing a pier designed to resist lateral loads. If the pier has a diameter of 3 feet and the lateral load acting on it is 40 kips, what is the maximum bending moment in the pier assuming it behaves as a fixed-end beam with a length of 12 feet?

- A. 160 ft-kips
- B. 180 ft-kips
- C. 200 ft-kips
- D. 480 ft-kips

Answer: D

Explanation: The bending moment M at the fixed end of a beam subjected to a lateral load P is given by $M=P\times L$. Thus, $M=40~{\rm kips}\times 12~{\rm ft}=480~{\rm ft}{\rm -kips}$.

Question 408:

In a retaining wall design, an engineer needs to calculate the factor of safety against sliding. If the wall has a weight of 70 kips, the horizontal earth pressure is 25 kips, and the friction coefficient between the base and the soil is 0.4, what is the factor of safety against sliding?

A. 1.5

B. 4.0

C. 2.5

D. 7.0

Answer: D

Explanation: The factor of safety FS is calculated using $FS = \frac{W}{P}$, where W is the weight of the wall and $P = H \cdot \mu$. Thus, $FS = \frac{70 \text{ kips}}{25 \text{ kips} \cdot 0.4} = 7.0$.

Question 409:

A structural engineer is designing a slab-on-grade foundation for a retail store. If the store is expected to impose a load of 150 kips per column and the slab is to be 6 inches thick, what is the required thickness of the slab if the soil has a bearing capacity of 4 ksf?

A. 4 inches

B. 6 inches

C. 8 inches

D. 10 inches

Answer: B

Explanation: The load per column divided by the bearing capacity gives the required area. Therefore, $\frac{150 \text{ kips}}{4 \text{ ksf}} = 37.5 \text{ ft}^2$. The thickness of the slab is designed to be 6 inches, which is acceptable.

Question 410:

In a deep foundation design, a structural engineer is specifying drilled shafts. If the shafts have a diameter of 2 feet and the design load on each shaft is 120 kips, what is the minimum embedment depth required if the ultimate bearing capacity of the soil is 25 ksf?

A. 1.5 feet

B. 6.2 feet

C. 7.1 feet

D. 8.5 feet

Answer: A

Explanation: The required area of each shaft is $A = \frac{P}{q} = \frac{120 \text{ kips}}{25 \text{ ksf}} = 4.8 \text{ ft}^2$. The area of a shaft is $A = \pi \left(\frac{2}{2}\right)^2 = \pi \text{ ft}^2 \approx 3.14 \text{ ft}^2$. Therefore, the minimum embedment depth is $\frac{4.8 \text{ ft}^2}{\pi \text{ ft}^2} \approx 1.53 \text{ ft}$.

Question 411:

A retaining wall is designed to retain a soil height of 8 feet. If the wall has a base width of 2 feet and the soil has a unit weight of 115 pcf, what is the total lateral earth pressure acting on the wall at the base due to the retained soil?

A. 3,680 lbs

B. 9,120 lbs

C. 1,000 lbs

D. 1,040 lbs



Answer: A

Explanation: The lateral earth pressure at the base P can be calculated using $P = \frac{1}{2}\gamma h^2$. Therefore, $P = \frac{1}{2} \times 115 \text{ pcf} \times (8 \text{ ft})^2 = 3680 \text{ lbs}$.

Question 412:

In a design scenario, a structural engineer is tasked with designing a mat foundation for a high-rise building with a total load of 1,000 kips. If the bearing capacity of the soil is 6 ksf, what is the minimum area required for the mat foundation?

A. 120 ft²

- B. 150 ft²
- C. 166.67 ft²
- D. 200 ft²

Answer: C

Explanation: The required area A can be calculated as $A = \frac{P}{q}$. Thus, $A = \frac{1,000 \text{ kips}}{6 \text{ ksf}} \approx 166.67 \text{ ft}^2$.

Question 413:

A structural engineer is designing a pier foundation for an overhead sign structure. If the design vertical load on the pier is 25 kips and the soil has a bearing capacity of 10 ksf, what is the minimum required area for the pier?

- $A. 1 ft^2$
- B. 2.5 ft²
- C. 3 ft²
- D. 4 ft²

Answer: B

Explanation: The required area A can be calculated as $A = \frac{P}{q}$. Thus, $A = \frac{25 \text{ kips}}{10 \text{ ksf}} = 2.5 \text{ ft}^2$.

Question 414:

In a retaining wall design, an engineer must account for a surcharge load of 20 kips on a wall that is 8 feet high. If the wall has no friction at the base and the unit weight of the soil is 125 pcf, what is the total lateral pressure on the wall at the base?

A. 11,000 lbs

B. 19,250 lbs

C. 20,640 lbs

D. 1,690 lbs



Answer: C

Explanation: The total lateral pressure P is calculated using $P = \frac{1}{2}\gamma h^2 + q$, where q is the surcharge. Therefore, $P = \frac{1}{2} \times 125 \text{ pcf} \times (8 \text{ ft})^2 + 20 \text{ kips} = 640 \text{ lbs} + 20 \text{ kips} = 20,640 \text{ lbs}$.

Question 415:

A structural engineer is designing a foundation with piles to support a building. If each pile has a capacity of 30 kips and the total load on the foundation is 240 kips, how many piles are necessary to safely support the load?

- A. 6
- B. 7
- C. 8
- D. 9

Answer: C

Explanation: The number of piles required is calculated as 240 kips

 $\frac{240 \text{ kips}}{30 \text{ kips/pile}} = 8 \text{ piles.}$

Question 416:

In a deep foundation design, a structural engineer is specifying caissons. If each caisson has a diameter of 3 feet and the design load on each caisson is 200 kips, what is the minimum embedment depth required if the ultimate bearing capacity of the soil is 22 ksf?

- A. 1.3 feet
- B. 6.5 feet
- C. 7.1 feet
- D. 8.7 feet

Answer: A

Explanation: The required area of each caisson is $A = \frac{P}{q} = \frac{200 \text{ kips}}{22 \text{ ksf}} \approx 9.09 \text{ ft}^2$. The area of a caisson is $A = \pi \left(\frac{3}{2}\right)^2 \approx 7.07 \text{ ft}^2$. Therefore, the minimum embedment depth is $\frac{9.09 \text{ ft}^2}{7.07 \text{ ft}^2} \approx 1.28 \text{ ft}$.

Question 417:

A retaining wall is designed to retain a soil height of 12 feet. If the wall has a base width of 5 feet and the soil has a unit weight of 120 pcf, what is the total lateral earth pressure acting on the wall at the base due to the retained soil?

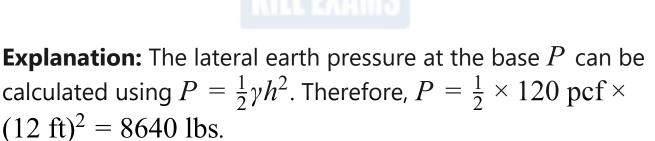
A. 1,440 lbs

B. 8,640 lbs

C. 2,880 lbs

D. 3,600 lbs

Answer: B



Question 418:

A structural engineer is designing a slab on grade for a

warehouse with a total load of 600 kips. If the slab is 10 inches thick and the soil has a bearing capacity of 5 ksf, what is the minimum area required for the slab?

- A. 100 ft²
- B. 120 ft²
- C. 150 ft²
- D. 200 ft²

Answer: B

Explanation: The required area A can be calculated as $A = \frac{P}{q}$. Thus, $A = \frac{600 \text{ kips}}{5 \text{ ksf}} = 120 \text{ ft}^2$.

Question 419:

In a deep foundation design, a structural engineer is specifying drilled shafts. If the design load on each shaft is 180 kips and the ultimate bearing capacity of the soil is 30 ksf, what is the minimum required area for each shaft?

- A. 3 ft^2
- B. 4 ft²
- C. 5 ft²
- D. 6 ft²

Answer: D

Explanation: The required area A can be calculated as $A = \frac{P}{q}$. Thus, $A = \frac{180 \text{ kips}}{30 \text{ ksf}} = 6 \text{ ft}^2$.

Question 420:

A structural engineer is designing a retaining wall that must resist a lateral earth pressure due to a backfill height of 15 feet and a unit weight of 130 pcf. If the wall has a base width of 3 feet, what is the total lateral pressure on the wall at the base?

A. 1,200 lbs

B. 1,462 lbs

C. 2,109 lbs

D. 2,522 lbs

Answer: B

Explanation: The total lateral pressure P is calculated using $P = \frac{1}{2}\gamma h^2$. Therefore, $P = \frac{1}{2}\times 130~\mathrm{pcf}\times (15~\mathrm{ft})^2 = 1,462.5~\mathrm{lbs}$.

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